

THREE KINDS OF POROSITY AND THEIR EFFECTS TO BENDING BEHAVIOR OF FUNCTIONALLY GRADED POROUS BEAMS BASED ON THE SIMPLE TIMOSHENKO BEAM THEORY

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Abstract: In this paper, the effects of three types of porosity to bending behavior of functionally graded porous (FGP) beams are studied. The finite element procedure is established and based on the simple Timoshenko beam theory. The results achieved in this paper are presented and compared with other results in the references to verify the feasibility in implementing the formula and writing the Matlab code. On the other hand, this paper can help researchers to have an overview of the bending behavior of the functionally graded porous beams.

Keywords: Bending behavior, Functionally graded porous (FGP) beam, Transverse displacement, Rotation, Simple Timoshenko beam

INTRODUCTION

Nowadays, functionally graded (FG) material has become one of the smart materials and it is used in many countries. From a mixture of ceramic and metal, it provided the continuous variation of material properties from the top surface to the bottom surface of structure. For example, some structures like nuclear tanks, spacecraft, etc. are produced based on the above material [1-3]. Due to the high applicability of functionally graded material, many studies related to various theories have been given to comment the mechanical behavior of functionally graded structures as [4-9]. However, porosity of the material can occur during the manufacturing process [10-12]. So, for a good knowledge of porosity effect on bending behavior of functionally graded structures, a study related to this issue must be considered as soon as possible. There are three types of structure like beam, plate and shell, but researchers are usually interested in beam structures because of its wide applications. Furthermore, many different beam theories were used to analyze beam structures like simple beam theory [13], classical beam theory [14, 15], first-order shear deformation theory [16-20] or higher-order shear deformation theory [21, 22]. However, using a simple Timoshenko beam model helps us to reduce the computational cost with the resulting error within the allowable range. On the other hand, beams made of functionally graded porous materials should be investigated as much as possible to help the designer have right knowledge about the mechanical properties. The few published papers on static bending behavior of FG beams can be listed here. Author Chen and co-workers presented the Ritz method to obtain the transverse bending deflections and critical buckling loads, where the trial functions take the form of simple algebraic polynomials [23]. A novel model was introduced for bending of functionally graded porous cantilever beams by [24] related to shape memory alloy/poroelastic composite material. In this article, authors verified the accuracy of the bending model by three-dimensional finite element procedure. Another paper

based on a trigonometric shear deformation theory was used to analyze the bending, vibration and buckling characteristics of functionally graded porous graphene-reinforced nanocomposite curved beams from [25], and so on. From above reasons, this paper is given to investigate the bending behavior of functionally graded porous beams. This paper has four parts. Part 1 gives the introduction as above. Part 2 presents the formulations as well as Part 3 shows some essential results. Finally, a few comments are also given in Part 4 respectively.

FORMULATIONS

A FGP beam with length L , width b and thickness h is considered. Three forms of porosity distributions are studied and shown in Fig. 1, in which (1) is uniform porous distribution and (2) and (3) are non-uniform porous distributions respectively.

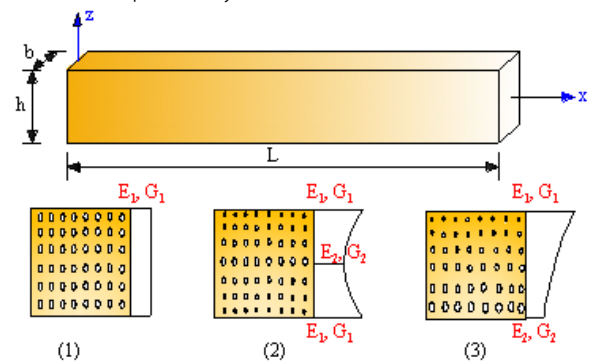


Figure 1. Functionally graded porous beam with three types of porosity 1, 2 & 3
The material properties $E(z)$ and $G(z)$ can be described as below

$$\begin{cases} E(z) = E_1(1 - e_0\chi) \\ G(z) = G_1(1 - e_0\chi) \end{cases} \quad \text{with} \quad (1)$$

$$\chi = \frac{1}{e_0} - \frac{1}{e_0} \left(\frac{2}{\pi} \sqrt{1 - e_0} - \frac{2}{\pi} + 1 \right)^2 \quad \text{for type (1)}$$

$$\begin{cases} E(z) = E_1 \left(1 - e_0 \cos \left(\frac{\pi z}{h} \right) \right) \\ G(z) = G_1 \left(1 - e_0 \cos \left(\frac{\pi z}{h} \right) \right) \end{cases} \quad \text{for type (2)} \quad (2)$$

$$\begin{cases} E(z) = E_1(1 - e_0 \cos(\frac{\pi z}{2h} + \frac{\pi}{4})) \\ G(z) = G_1(1 - e_0 \cos(\frac{\pi z}{2h} + \frac{\pi}{4})) \end{cases} \text{ for type (3)} \quad (3)$$

The porosity coefficient e_0 must satisfy $0 < e_0 < 1$ and

$$e_0 = 1 - \frac{E_2}{E_1} = 1 - \frac{G_2}{G_1} \quad (4)$$

Based on finite element method (FEM), the degrees of freedom associated with a node of a simple Timoshenko beam element are a transverse displacement and a rotation as depicted in Fig. 2. Using the principles of simple beam theory, the beam element stiffness matrix will be derived

$$K_e = \frac{E_e I_e}{L_e^3 (1 + \Phi)} \begin{bmatrix} 12 & 6L_e & -12 & 6L_e \\ 6L_e & (4 + \Phi)L_e^2 & -6L_e & (2 - \Phi)L_e^2 \\ -12 & -6L_e & 12 & -6L_e \\ 6L_e & (2 - \Phi)L_e^2 & -6L_e & (4 + \Phi)L_e^2 \end{bmatrix} \quad (5)$$

with

$$\Phi = \frac{12E_e I_e}{G_e k A_e L_e^2} \quad (6)$$

and $k=5/6$ is called the shear correct factor.

According to the principle of minimum total potential energy, the element equation can be described as

$$\frac{E_e I_e}{L_e^3 (1 + \Phi)} \begin{bmatrix} 12 & 6L_e & -12 & 6L_e \\ 6L_e & (4 + \Phi)L_e^2 & -6L_e & (2 - \Phi)L_e^2 \\ -12 & -6L_e & 12 & -6L_e \\ 6L_e & (2 - \Phi)L_e^2 & -6L_e & (4 + \Phi)L_e^2 \end{bmatrix} \begin{Bmatrix} w_i \\ \varphi_i \\ w_j \\ \varphi_j \end{Bmatrix} = \begin{Bmatrix} f_i \\ m_i \\ f_j \\ m_j \end{Bmatrix} \quad (7)$$

After assembly, the bending parameters can be obtained by solving the following equation

$$\mathbf{Kd} = \mathbf{F} \quad (8)$$

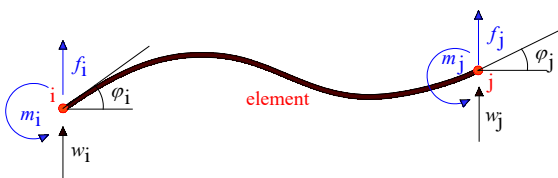


Figure 2. The simple Timoshenko beam element

By using three letters 'C', 'S' and 'F' refer to the clamped, simply supported and free condition, all boundary conditions can be revealed as below

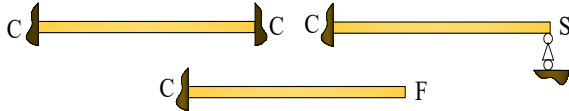


Figure 3. Three types of beam

$$(CS) w(0) = \varphi(0) = 0, w(L) = 0 \quad (9)$$

$$(CC) w(0) = \varphi(0) = 0, w(L) = \varphi(L) = 0 \quad (10)$$

$$(CF) w(0) = \varphi(0) = 0 \quad (11)$$

More clearly, the finite element system of equations can be reached as below:

- ≡ Input data: geometric data and material properties.
- ≡ Calculating constitutive matrix.
- ≡ Loop over elements: calculating element stiffness matrix and element force vector.

- ≡ Assembling all parts in the global coordinate system
- ≡ Applying boundary conditions
- ≡ Solving equation for static bending
- ≡ Display transverse displacements and rotations at nodes of system.

NUMERICAL EXAMPLES

Firstly, the validity of the proposed model is checked for (CC) and (CS) isotropic beams under a uniform load $q = 10^6 \text{ N/m}^2$. The material and geometric properties are $E = 1 \text{ GPa}$, $\nu = 1/3$, $b = 0.1 \text{ m}$, $h = 0.1 \text{ m}$ and $L = 10h$. The maximum transverse displacement and rotation as in Table 1 are calculated and compared with analytical solutions [26] as follows:

$$w = \frac{1}{EI} \left[\frac{1}{24} qL^2 x^2 - \frac{1}{12} qLx^3 + \frac{1}{24} qx^4 \right] \quad (12)$$

$$\varphi = \frac{1}{EI} \left[\frac{1}{12} qL^2 x - \frac{1}{4} qLx^2 + \frac{1}{6} qx^3 \right] \quad (13)$$

$$w = \frac{1}{48EI} \left[-3qL^2 x^2 + 5qLx^3 - 2qx^4 \right] \quad (14)$$

$$\varphi = \frac{1}{48EI} \left[-6qL^2 x + 15qLx^2 - 8qx^3 \right] \quad (15)$$

Table 1. The comparison of the maximum transverse displacements at position $x = L/2$ of (SS) isotropic beams with $L/h = 5$

CC	w_{\max}		φ_{\max}	
	Analytical	Paper	Analytical	Paper
	0.3125	0.3126	0.9375	0.9383
CS	w_{\max}		φ_{\max}	
	Analytical	Paper	Analytical	Paper
	0.6480	0.6466	1.7187	1.7002

It can be seen that the results obtained from the paper are completely approximate with other results. The relative error among above results can be explained by using different approaches.

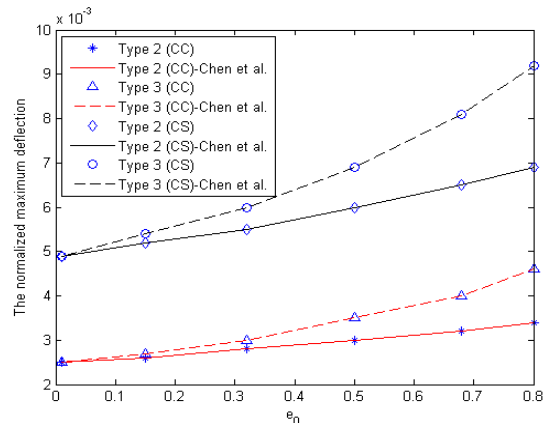


Figure 4. Convergence of the deflection

Secondly, the material of the porous beam is assumed to be steel foam with $E_0 = 200 \text{ GPa}$, $\nu = 1/3$. The cross section of beam is $h = 0.1 \text{ m}$, $b = 0.1 \text{ m}$. The normalized maximum deflections $\bar{w} = w_{\max} / h$ based on this study for two boundary conditions (CC) and (CS) are compared with other results of [23] as in Figure 4. Again, their convergence proves the reliability of the proposed method in bending analysis of functionally graded porous beams.

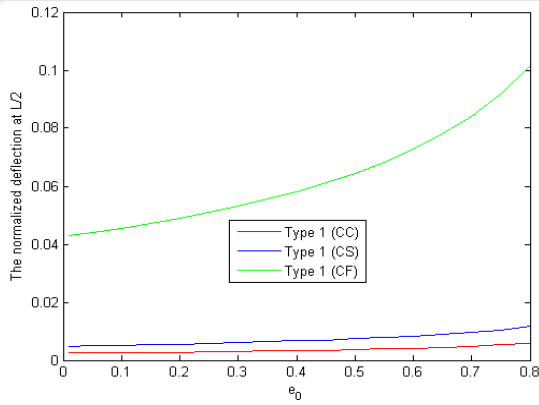


Figure 5. The influence of e_0 on the deflection of porous beam with type 1 for three boundary conditions

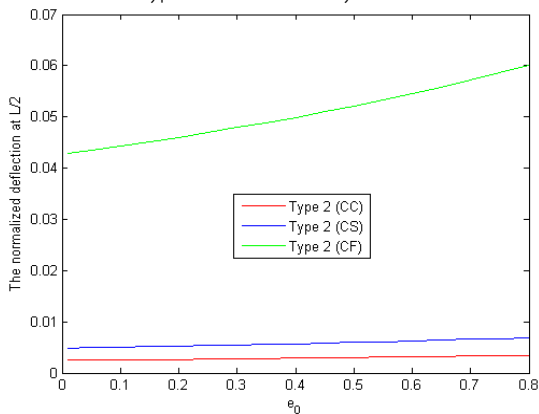


Figure 6. The influence of e_0 on the deflection of porous beam with type 2 for three boundary conditions

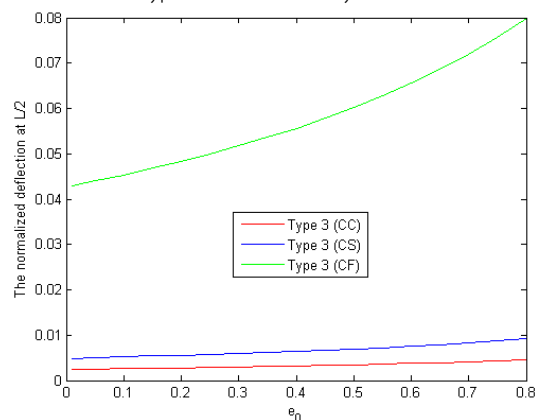


Figure 7. The influence of e_0 on the deflection of porous beam with type 3 for three boundary conditions

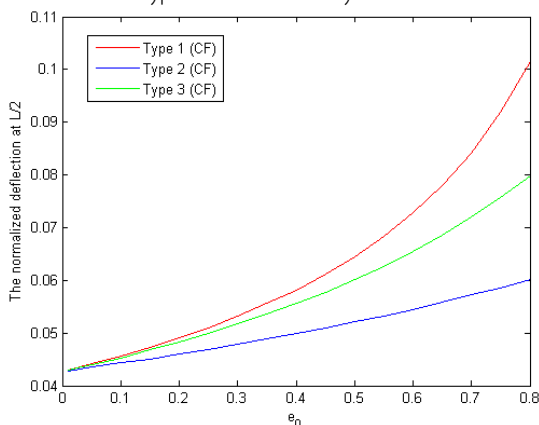


Figure 8. The influence of e_0 on the deflection of (CF) porous beam with three types 1, 2 & 3

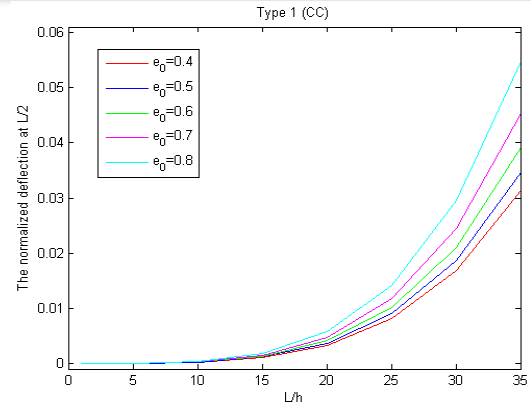


Figure 9. The deflections of type 1 (CC) porous beam by changing ratio L/h and porosity factor e_0

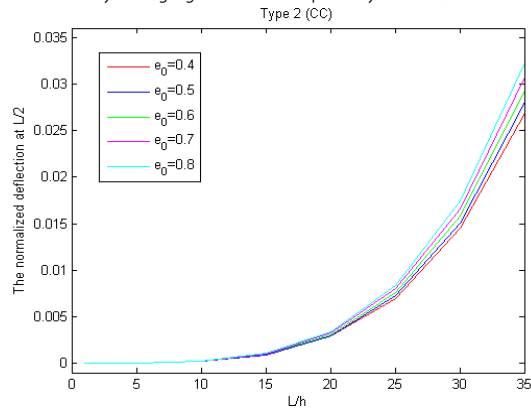


Figure 10. The deflections of type 2 (CC) porous beam by changing ratio L/h and porosity factor e_0

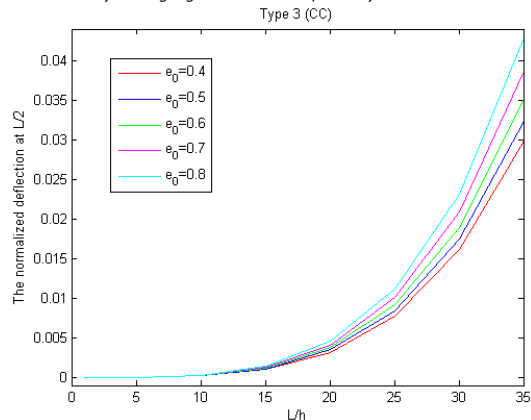


Figure 11. The deflections of type 3 (CC) porous beam by changing ratio L/h and porosity factor e_0

Thirdly, by changing the boundary condition from (CC) to (CS) and (CF), the bending behaviors of FGP beams can be seen in Figures 5-7 for three types 1, 2 & 3. Once again, the effects of porosity on the bending behavior of this structure are clearly presented in these figures. Furthermore, Figure 8 depict the influence of porosity on the deflections of (CF) porous beams for type 1, type 2 and type 3 respectively. Finally, by varying the porosity coefficient e_0 , the length to thickness ratio L/h and three types 1, 2 & 3, the results of the normalized transverse displacement $\bar{w} = w(L/2)/h$ at position $L/2$ of FGP beams with (CC) boundary condition are plotted in Figure 9-11. As the porosity value increases, the deflection of FGP beam also increases and this statement holds for all cases.

CONCLUSION

In the paper, the bending behaviors of functionally graded porous (FGP) beams under three different types of boundary condition and three kinds of porosity are presented. The verification results of this paper are in good agreement with other results in reference. The topic and approach of the paper are simple, the main aim of the author is to affirm the applicability of the simple beam theory to analyze the functionally graded porous (FGP) beams with acceptable results.

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